

DEFORMATIONS OF SOIL MASSES UNDER THE ACTION OF HUMAN-INDUCED FACTORS

Mykola Kuzlo¹, Viktor Moshynskyi², Nataliia Zhukovska³, Viktor Zhukovskyy³

¹National University of Water and Environmental Engineering, Department of Highways, Bases and Foundations, Rivne, Ukraine,

²National University of Water and Environmental Engineering, Department of Land Management, Cadastre, Land Monitoring and Geoinformatics, Rivne, Ukraine,

³National University of Water and Environmental Engineering, Department of Computer Sciences and Applied Mathematics, Rivne, Ukraine

Abstract. Significant changes in the stress-strain state cause a change in the soil profile of the massif, which is affected by various physical and chemical factors. In particular, groundwater filtration, mass transfer, heat transfer, dissolution and leaching of soil masses. This can lead to various types of accidents. Therefore, the study of the stress-strain state of the soil massif is an important topic. Nonlinear dependences in the form of polynomials of the modulus of deformation and Lamé coefficients on the concentration of salt solutions and their temperature have been received in this research based on experimental research and their statistical processing. This allowed improving the mathematical model of the stress-strain state of the soil taking into account the nonlinear deformation processes occurring in the soil masses under the presence and filtration of saline solutions in non-isothermal conditions.

Keywords: mathematical models, statistical analysis, stress-strain state, deformation

DEFORMACJE MASY GLEBY POD DZIAŁANIEM CZYNNIKÓW CZŁOWIEKA

Streszczenie. Znaczące zmiany stanu naprężeniowo-odkształceniowego powodują zmianę profilu glebowego masywu, na który wpływają różne czynniki fizyczne i chemiczne. W szczególności filtracja wód gruntowych, przenikanie masy, przenoszenie ciepła, rozpuszczanie i wymywanie mas glebowych. Może to prowadzić do różnego rodzaju wypadków. Dlatego ważnym tematem jest badanie stanu naprężeniowo-odkształceniowego masywu glebowego. W niniejszych badaniach otrzymano nieliniowe zależności w postaci wielomianów modułu odkształcenia i współczynników Lamé'a od stężenia roztworów soli i ich temperatury w oparciu o badania eksperymentalne i ich obróbkę statystyczną. Pozwoliło to na udoskonalenie modelu matematycznego stanu naprężeniowo-odkształceniowego gruntu uwzględniającego nieliniowe procesy odkształceń zachodzących w masach gruntu pod wpływem obecności i filtracji roztworów soli w warunkach nieizotermicznych.

Słowa kluczowe: modele matematyczne, analiza statystyczna, stan naprężenia-odkształcenia, odkształcenie

Introduction

Human activity leads to changes in hydrogeological conditions [5, 12]. And the change in hydrogeological conditions on Earth is accelerating. The reason for such changes is the excessive strengthening of a previously unknown geological agent. This new geological agent is human activity. In the research paper [19], human technical activity is considered a driving geological force that not only changes the face of the earth's surface but also makes significant changes in the upper part of the earth's crust, which is comparable to scale and consequences with geological processes.

The problem is greatly complicated if the soil is contaminated with various solutions of industrial facilities' slurry ponds, which accumulate industrial waste. These objects often reach quite large sizes, and they constantly or periodically get a large amount of so-called "spent" liquids used in technological processes, which are characterized by high mineralization and temperature.

Currently, the data have been used which are developed for soil mechanics to assess the soil mass deformation which is saturated with saline solutions [8, 9]. And this is the subject to the action of low-concentration natural groundwater [4].

Also some researchers are working on geotechnical issues of landslides in Ukraine [11] and decreasing service life of buildings under the regular explosion loads [10]. At the same time others proved the theory that mining activities always have and exclusively caused considerable changes in the environment [7]. Moreover, the rapid growth of population, urban planning, agricultural and industrial sectors also may have a significant effect [2].

Scientists pay attention also to asphalt mixes. In paper [3] they developed mathematical models for not isothermal conditions of marginal soils deformation (foamed and emulsified sulfur asphalt soils mixes) using specific software. Paper [1] provides a new numerical model in discrete element analysis for simulating flow time and number tests of asphalt mixes.

However, our previous experiments have shown, the compressibility of soils subject to the action of saline solutions depends on many factors. It depends from the degree of load and the concentration of saline solutions and their temperature as well [13, 14, 17].

Human-induced impact on soil masses leads to various factors: hydrodynamic forces of the filtration flow, changes in the filtration and deformation characteristics of the soil and so on. The change of these factors can be important, which leads to significant deformations of the earth's surface [6, 21, 23]. These deformations complicate regular operation and, in some cases, lead to accidents of industrial and energetic objects and can cause significant economic damage and even victims. In difficult military times, the issue of the stress-deformation state of soil massifs also acquires new relevance. This especially applies to areas of hostilities affected by massive artillery strikes.

The work aims to study the influence of the concentration of salt solutions and their temperature on the deformation processes of the soil and to create on this basis a computational-theoretical apparatus for forecasting the deformations of soil masses.

1. Physical experiments

To establish the influence of solutions' concentration and their temperature on the deformation characteristics of the soil, experimental studies were conducted in the geotechnical laboratory of the National University of Water and Environmental Engineering (Rivne, Ukraine).

The experiments were performed on a compression-filtration device according to state standard methods [18]. Issued in January 1991 in Kyiv, this standard outlines the specific methods to accurately assess these soil properties, which are crucial for construction and engineering purposes. The standard ensures that soil testing is conducted consistently and reliably, providing essential data for evaluating soil behavior under various conditions, which is vital for the design and safety assessment of construction projects.

Soil samples of disturbed structures with specified values of density and humidity were used for analysis. The soils for the study were sandy clays with the number of plasticity $I_p = 7.0\%$, the porosity coefficient $e = 0.55$. Soil pastes for the experiment had been saturating with NaCl saline solutions' concentration 0; 22; 44; 65; 90; 110; 130; 145; 165 g/l for 2 days. The temperature value was taken in the range from 22°C to 88°C.

Results of experimental data have been shown in tables 1–3.



Table 1. The value of the deformation modulus $E(c, T)$

c, g/l	Deformation modulus $E(c, T)$, kPa			
	$T = 22^\circ\text{C}$	$T = 31^\circ\text{C}$	$T = 60^\circ\text{C}$	$T = 88^\circ\text{C}$
0	6555	6003	5692	5117
22	5520	5290	5175	4743
44	5002	4887	4738	4427
65	5060	4773	4427	4140
90	5164	4876	4485	4025
110	5348	4922	4571	4111
130	5474	5060	4715	4312
145	5681	5232	4916	4571
165	6038	5462	5175	4830

Table 2. The value of Lamé parameter $\lambda(c, T)$

c, g/l	Lamé parameter $\lambda(c, T)$, kPa			
	$T = 22^\circ\text{C}$	$T = 31^\circ\text{C}$	$T = 60^\circ\text{C}$	$T = 88^\circ\text{C}$
0	5663	5187	4918	4422
22	4770	4571	4471	4098
44	4322	4222	4094	3826
65	4372	4124	3826	3577
90	4462	4213	3875	3478
110	4620	4252	3950	3552
130	4729	4372	4074	3726
145	4909	4521	4248	3950
165	5217	4719	4471	4173

Table 3. The value of Lamé parameter $\mu(c, T)$

c, g/l	Lamé parameter $\mu(c, T)$, kPa			
	$T = 22^\circ\text{C}$	$T = 31^\circ\text{C}$	$T = 60^\circ\text{C}$	$T = 88^\circ\text{C}$
0	2427	2222	2107	2107
22	2043	1959	1915	1756
44	1852	1810	1754	1638
65	1874	1767	1638	1532
90	1912	1805	1660	1490
110	1980	1822	1692	1522
130	2026	1874	1745	1596
145	2104	1937	1820	1692
165	2235	2022	1915	1788

2. Results and discussion

The following dependences of the Young's deformation modulus and Lamé parameters on the concentration of saline solutions and temperature were obtained as a result of experimental data statistical mathematical processing:

$$E(c, T) = (a_{18} \cdot c^2 + a_{17} \cdot c + a_{16}) \cdot T^2 + (a_{15} \cdot c^2 + a_{14} \cdot c + a_{13}) \cdot T + a_{12} \cdot c^2 + a_{11} \cdot c + a_{10} \quad (1)$$

where

$$a_{18} = 1.179 \cdot 10^{-5}, \quad a_{17} = -7.755 \cdot 10^{-4}, \quad a_{16} = 0.024, \quad a_{15} = -1.713 \cdot 10^{-3}, \\ a_{14} = 0.138, \quad a_{13} = -17.539, \quad a_{12} = 0.198, \quad a_{11} = -30.741, \\ a_{10} = 6.546 \cdot 10^3;$$

$$\lambda(c, T) = (a_{28} \cdot c^2 + a_{27} \cdot c + a_{26}) \cdot T^2 + (a_{25} \cdot c^2 + a_{24} \cdot c + a_{23}) \cdot T + a_{22} \cdot c^2 + a_{21} \cdot c + a_{20} \quad (2)$$

where

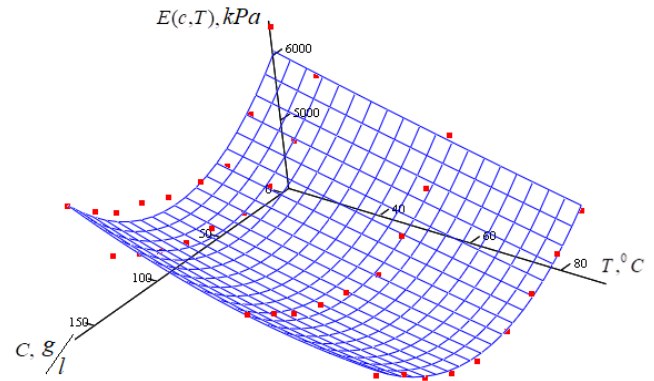
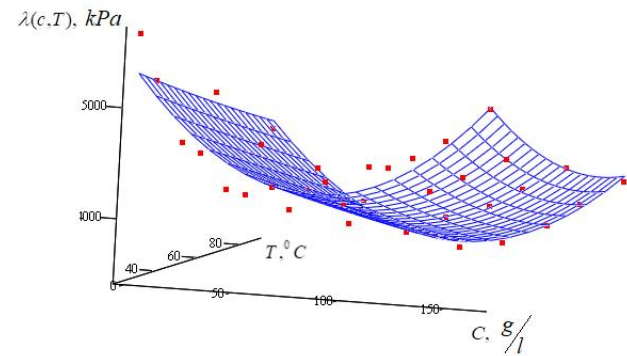
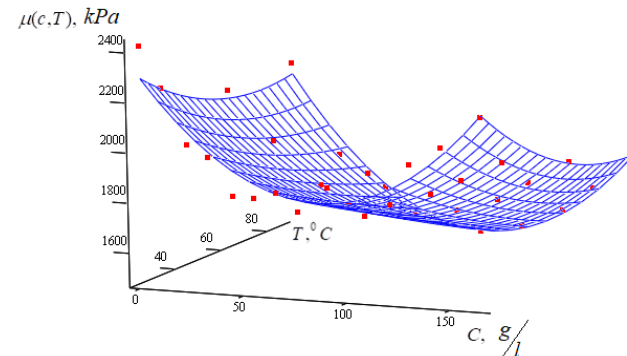
$$a_{28} = 1.177 \cdot 10^{-5}, \quad a_{27} = -9.23 \cdot 10^{-4}, \quad a_{26} = 0.018, \quad a_{25} = -1.558 \cdot 10^{-3}, \\ a_{24} = 0.128, \quad a_{23} = -14.343, \quad a_{22} = 0.166, \quad a_{21} = -25.543, \\ a_{20} = 5.611 \cdot 10^3;$$

$$\mu(c, T) = (a_{38} \cdot c^2 + a_{37} \cdot c + a_{36}) \cdot T^2 + (a_{35} \cdot c^2 + a_{34} \cdot c + a_{33}) \cdot T + a_{32} \cdot c^2 + a_{31} \cdot c + a_{30} \quad (3)$$

where

$$a_{38} = 1.242 \cdot 10^{-5}, \quad a_{37} = -1.983 \cdot 10^{-3}, \quad a_{36} = 0.084, \quad a_{35} = -1.3 \cdot 10^{-3}, \\ a_{34} = 0.191, \quad a_{33} = -12.748, \quad a_{32} = 0.085, \quad a_{31} = -13.816, \\ a_{30} = 2.533 \cdot 10^3.$$

The results of experimental data mathematical processing with the choice of the optimal scale have been shown in figures 1–3.

Fig. 1. The graphic of Young's deformation modulus $E(c, T)$, kPa on pollution concentration and temperatureFig. 2. The graphic of Lamé's parameter dependency $\lambda(c, T)$, kPa on pollution concentration and temperatureFig. 3. The graphic of Lamé's parameter dependency $\mu(c, T)$, kPa on pollution concentration and temperature

The established dependences of the Lamé coefficients $\lambda(c, T)$ and $\mu(c, T)$ on the filtration solution concentration and its temperature are in the area of scientific and practical interest. It can be further used in the creation of underground hydromechanics mathematical models and estimation of the stress-deformed state of soil environments. Accordingly, a new scientific field of these dependencies is emerging in new and updated mathematical models.

2.1. Mathematical model

Let us consider a one-dimensional problem for determining the vertical displacements of the soil layer based on the problem complexity. The soil mass thickness l has been given, in which the processes of heat transfer and mass transfer of contaminated substances take place (Fig. 4).

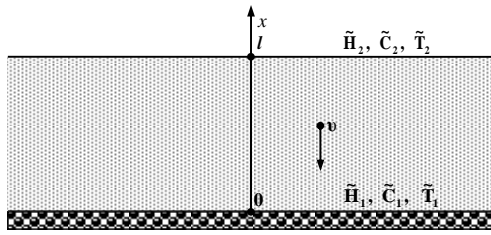


Fig. 4. Soil mass scheme in one-dimensional case

On Fig. 4. there is the scheme of soil mass in the one-dimensional case under the existence of pressures $\tilde{H}_2 > \tilde{H}_1$, the concentration of contaminated substances $\tilde{C}_2 > \tilde{C}_1$ and temperature $\tilde{T}_2 > \tilde{T}_1$ in the upper and lower ones of its limits.

The mathematical model of the one-dimensional problem of the stress-deformed state in the layer and soil mass thickness l with the spread of contaminated substances and nonisothermal conditions in conventional symbols can be described in the following boundary value problem [16, 20, 22]:

$$(\lambda + 2\mu) \frac{d^2U}{dx^2} + \frac{d(\lambda + 2\mu)}{dx} \frac{dU}{dx} - \left(\frac{d(\lambda + 2\mu)}{dx} T + (\lambda + 2\mu) \frac{\partial T}{\partial x} \right) \alpha_T = X \tag{4}$$

$$\varepsilon = \frac{dU}{dx}, \sigma = E \left(\frac{dU}{dx} - \alpha_T \bar{T} \right), x \in (0, l), \tag{5}$$

$$L_1 U(0) = 0, L_2 U(l) = 0, \tag{6}$$

and equations describing convective diffusion taking into account heat and mass transfer and convective heat and mass transfer with appropriate boundary conditions.

Here x – vertical coordinate, $x \in (0; l)$; $U(x)$ – transfer in soil mass relative to the axis Ox ; X – volumetric force, which is determined with the formula $X = \gamma_{sb} + \frac{dp}{dx}$; λ, μ – Lamé parameters; $c(x, t)$ – concentration of salt solution in soil mass; α_T – coefficient of linear heat expansion; $T(x, t)$ – temperature; γ_{sb} – specific soil gravity in weighed state; E – Young’s module; $\varepsilon(x), \sigma(x)$ – deformations and strains in the layer of soil mass relatively.

A finite-difference approximation of equations (4)–(6) by the finite difference method has been performed. The method of multiple calculations has been used to find the values of vertical displacements of the soil mass from equation (4).

The numerical experiments have been performed to establish the effect on the soil mass stress-deformed state of changes in the contaminated substance’s temperature, in the presence of this factor, and in its absence.

For the case when the temperature of the pollutants corresponds to the environment, the dependences of the Lamé parameters and the Yung’s modulus on the concentration of the pollutants have been accepted based on the experimental data which are given in [15]:

$$\lambda(c) = a_3^2 \cdot c^3 + a_2^2 \cdot c^2 + a_1^2 \cdot c + a_0^2 \tag{7}$$

where $a_3^2 = -1798.96, a_2^2 = 4314.732, a_1^2 = -2615.37, a_0^2 = 2545.743;$

$$\mu(c) = a_3^3 \cdot c^3 + a_2^3 \cdot c^2 + a_1^3 \cdot c + a_0^3 \tag{8}$$

where $a_3^3 = -1205.28, a_2^3 = 2880.321, a_1^3 = -1741.92, a_0^3 = 1696.324;$

$$E(c) = a_3^4 \cdot c^3 + a_2^4 \cdot c^2 + a_1^4 \cdot c + a_0^4 \tag{9}$$

where $a_3^4 = -0.000393, a_2^4 = 0.1878866, a_1^4 = -22.70202, a_0^4 = 4410.552.$

Numerous experiments have been carried out for the following input data:

$$l = 10 \text{ m}, \gamma_{sb} = 1.1 \cdot 10^4 \text{ Pa/m}, \alpha_T = 1 \cdot 10^{-6} \text{ 1/deg}$$

As soil environments can be affected by temperature, the effect of temperature and the concentration of contaminants on the values of vertical displacements must be taken into account.

The results of numerous experiments have been highlighted in Fig. 5.

This chart was received in our developed software complex. This software complex has been created as a web platform for online mathematical and computer modeling. It consists of the backend and frontend parts. The backend was written in C# programming language and used ASP.NET. Core technology, while the frontend uses Material framework and communicate with backend via specific API functions.

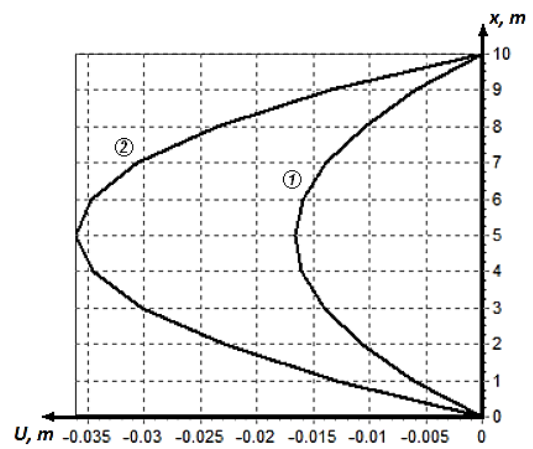


Fig. 5. The graphics of transfer distribution in the conditions of heat mass transfer under the filtration of salt solutions in nonisothermal conditions for $t = 1440$ days: 1 – under $\lambda = \lambda(c), \mu = \mu(c)$; 2 – under $\lambda = \lambda(c, T), \mu = \mu(c, T)$

Experimental studies have shown that considering the concentration of pollutants reduces the values of Lamé parameters compared to not considering the concentration. Taking into account the dependencies of the Lamé coefficients on the concentration of pollutants and temperature, as seen in Fig. 5, displacement values increase compared to displacement values considering only the dependence of the Lamé coefficients on concentration.

3. Conclusion

The obtained dependences of the deformation modulus and Lamé coefficients in the form of polynomials on the concentration of saline solutions and their temperature allowed to improve mathematical models of the stress-deformed state of the soil taking into account nonlinear deformation processes occurring in soil masses under the presence and filtration of saline solutions.

Incorporating the effect of temperature on soil environments, it’s crucial to consider how both temperature and contaminant concentration influence vertical displacement values. This aspect is particularly important when modeling soil behavior under varied environmental conditions. Additionally, in scenarios where pollutant temperatures align with the ambient environment, understanding the relationship between pollutant concentration and soil properties like Lamé parameters and Young’s modulus becomes vital. These relationships are key to accurately predicting soil response and ensuring the integrity of structures built on or within these soils.

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Prof. Mykola Kuzlo

e-mail: m.t.kuzlo@nuwm.edu.ua

He is an expert in hydro-melioration, holding a Dr. Sc. in Technical Sciences (2015) and a Professorship (2016). He graduated from the Ukrainian Institute of Water Management Engineers in Rivne (1984) and has been working there since 1989. He serves as the Head of the Department of Automobile Roads, Foundations, and Bases. His research focuses on predicting soil mass deformations, evaluating the stability of soil slopes, and assessing natural slopes' stability under changing hydrogeological conditions and anthropogenic factors.



<https://orcid.org/0000-0002-1016-0396>

Prof. Viktor Moshynskyi

e-mail: v.s.moshynskyi@nuwm.edu.ua

He is the Rector of the National University of Water and Environmental Engineering. With a career starting in 1987 as a hydrogeologist, he earned a Doctor in Agricultural Sciences in 2002. His academic career includes roles as an Associate Professor in agrochemistry, soil science, and agriculture, and later as Head of the Department of Land Management and Cadastre. Research interests: environmental protection, agricultural melioration, land management, and mathematical modeling in agriculture and environmental activities.



<https://orcid.org/0000-0002-1661-6809>

Ph.D. Eng. Nataliia Zhukovska

e-mail: n.a.zhukovska@nuwm.edu.ua

She is an associate professor at the Department of Computer Science and Applied Mathematics. She earned her Ph.D. in Technical Sciences in 2016 with a dissertation on "Mathematical Modeling of Heat and Mass Transfer in Filtration of Salt Solutions and their Effect on Deformation Processes in Soil Masses". N. Zhukovska specializes in mathematical modeling and computational methods, with over 70 scientific and methodological works. She is a laureate of the 2018 Young Scientists Award from the Rivne Regional State Administration.



<https://orcid.org/0000-0001-7839-0684>

Ph.D. Eng. Viktor Zhukovskyy

e-mail: v.v.zhukovskyy@nuwm.edu.ua

He is an associate professor at the Department of Computer Science and Applied Mathematics. He earned his Ph.D. in Mathematical Modeling and Computational Methods in 2018 with his dissertation on "Mathematical and Computer Modeling of Mass Transfer of Salt Solutions in Catalytic and Dispersed Media of Microporous Structure". He is a laureate of the 2018 Young Scientists Award from the Rivne Regional State Administration.



<https://orcid.org/0000-0002-7088-6930>